# THREE-DIMENSIONAL STATIC ANALYSIS OF MARINE CABLES 

Somchai Chucheepsakul Department of Civil Engineering King Mongkut's Institute of Technology-Thonburi Bangkok, Thailand

Silpachai Subwonglee
Highway Department, Ministry of Comminications
Bangkok, Thailand

# Three-dimensional static analysis of marine cables 

Somchai Chuchecpsakul<br>Department of Civil Engineering, King Mongku's Institute of Technology, Thonburi, Bangkok, Thailand<br>Silpachai Subwonglee<br>Highway Department, Ministry of Communications, Bangkok, Thailand

ABSTRACT: A method to determine the static equilibrium configuration of marine cables in three - dimensional space is presented. In present approach, the known quantities are the top tension and the location at both ends of the cable while the total arc length and equilibrium configuration are to be determined. A functional is introduced in which the potential energy of stretching and virtual work done due to current loadings are included. A finite element method was developed to solve the problem and a numerical example is given.

## 1. INTRODUCTION

Marine cables deployed in deep water can be subjected to cross current which results in three dimensional configuration. The paper concerning three dimensional analysis of cable are found in Refs.[3, 4, 5, 6\}. Most of the analysis assumed the total arc length as a known quantity, while the top tension is an unknown. In many cases the magnitude of top tension is specified either by the strength of cable or the lifting capacity. Thus, the problem of static analysis for the cable is to find the cable configuration and the total arc length when the top tension is given. Figure la shows such cable configuration in three dimensional upuce.

A variational method used previously for two dimensional cases (Ref. (2)) is extended to the present problem. Three dependent variables are used: the two horizontal coordinates of the centroidal line and the cable tension. To obtain these three unknowns, two variational equations and one equilibrium equation are used. An example of undersea cable given by Ref.[3] is used for demonstration.
2. EQUILIBRIUM EQUATIONS

The equilibrium equations for an infinitesimal segment of cable (Fig.lb) are $(1,3)$

$$
\begin{align*}
& \frac{d T}{d s}=W_{e} \sin \phi-P_{t}  \tag{1}\\
& \frac{d a}{d s}=-P_{b} / T \cos \phi  \tag{2}\\
& \frac{d d}{d s}=\left(W_{e} \cos \phi-P_{n}\right) / T \tag{3}
\end{align*}
$$

In which $T$ is tension in cable, $W$ is the effective weight of cable, $\phi$ is the angle between the horizontal plane and the tangent vector $\hat{i}$ and $\theta$ is the angle petween the projection of the tangent vector $\hat{t}$ in the horizontal plane. The geometric relation of the differential segment of cable is given by

$$
\frac{d x}{d s}=\cos \psi \cos \vartheta, \quad \frac{d y}{d \theta}=\cos \psi \sin \theta, \quad \frac{d z}{d s}=\sin \psi
$$

$(4 a, 4 b, 4 c)$


Fig. la Cable in equilibrium

The external forces $P_{i}, P_{b}$ and $P$ are the drag force due to current velocity in the horizontal plane which can be expressed ds

$$
\begin{align*}
& P_{t}=\frac{1}{2} P_{w} C_{D t} \cap D\left(V_{x} \cos \psi \cos \theta+v_{y} \cos \psi \sin \theta\right)^{2}  \tag{5}\\
& P_{b}=\frac{1}{2} \rho_{w} C_{D n} D\left(-v_{x} \sin \theta+v_{y} \cos \theta\right)^{2}  \tag{6}\\
& P_{n}=\frac{1}{2} \rho_{w} C_{D n} D\left(-v_{x} \sin \phi \cos \theta-v_{y} \sin \psi \sin \theta\right)^{2} \tag{7}
\end{align*}
$$

where $\rho_{W}$ is the density of sea water, $C_{D t}$ and $C_{D n}$ are tangential and normal drag coefficient, $v_{x}$ and $v_{y}$ are the components of current velocity in the horizontal plane.

Six independent variables $T, x, y, z, \phi$, and $\theta$ in Eqs. (1)-(4) are used to define any point $s$ in the cable. These six equations can be solved numerically [3] with specified boundary conditions and cable length. The equilibrium configuration as well as tension along the cable length can be obtained. However if the cable length is not known at the beginning, the complexity arises and it is difficult to solve these equations. This difficulty can be overcome by introducing the variational formulation presented herein.
3. VARIATIONAL FORMULATION

An energy functional for analyzing marine cables in three dimensions can be written as

$$
\begin{equation*}
\pi=\int_{0}^{H}\left(T \sqrt{1+x^{\prime 2}+y^{\prime 2}}-p_{x} \cdot x-p_{y} \cdot y\right) d z \tag{8}
\end{equation*}
$$

where $P_{X}$ and $P_{y}$ are the distributed external forces per unit arc length acting along $x$ and $y$ directions respectively. These forces are defined as

$$
\begin{equation*}
P_{x}=\left(P_{t x}+P_{b x}+P_{n x}\right) / s i n \phi \tag{9}
\end{equation*}
$$

$$
\begin{equation*}
P_{y}=\left(P_{t y}+P_{b y}+P_{n y}\right) / \sin \phi \tag{10}
\end{equation*}
$$

in which $P_{t x}, P_{t y}, P_{b x}, P_{b y}, P_{n x}$ and $P_{n y}$ are the components of $P_{n}, P_{b}$ and $P_{t}$ in
 rite $P_{x}$ and $P_{y}$ as follows

$$
\begin{align*}
P_{x}= & \frac{1}{2} \rho_{w} D\left\{C_{D t} \pi\left(v_{x} \cos \psi \cos \theta+v_{y} \cos \phi \sin \theta\right)^{2} \cos \phi \cos \theta\right. \\
& +C_{D n}\left(-v_{x} \sin 0+v_{y} \cos \theta\right)^{2}(-\sin \theta) \\
& \left.+C_{D n}\left(-v_{x} \sin \psi \cos \theta-v_{y} \sin \phi \sin ()\right)^{2}(-\sin \phi \cos \theta)\right\} / \sin \phi  \tag{11}\\
P_{y}= & \frac{1}{2} \rho_{w} D\left\{C_{D t} \pi\left(v_{x} \cos \phi \cos 0+v_{y} \cos \psi \sin \theta\right)^{2} \cos \phi \sin \theta\right. \\
& +C_{D n}\left(-v_{x} \sin \theta+v_{y} \cos \theta\right)^{2} \cos \theta \\
& +C_{D n}\left(-v_{x} \sin \psi \cos \theta-v_{y} \sin (\sin \theta)^{2}(-\sin \phi \sin \theta)\right\} / \sin \phi \tag{12}
\end{align*}
$$

An expression for the cable tension at any position is obtained by integrating Eq.(1) from location $z$ to the top end and using Eq.(4c), one gets

$$
\begin{equation*}
T=T_{H}+W_{e}(z-11)-\int_{z}^{H} \frac{P_{t}}{\sin \psi} d z \tag{13}
\end{equation*}
$$

Hormally $P_{t}$ is small, then cable tension reduces to

$$
\begin{equation*}
T=T_{H}+W_{e}(Z-H) \tag{14}
\end{equation*}
$$

were $T_{H}$ is the top tension and assumed to be known and $H$ is the total depth.
4. numerical procedure

Three unknowns to be involved are $x(z), y(z)$ and $T(z)$. The stationary condition $0 \boldsymbol{\pi}=0$, which means vanishing of the variation due to $x$ and the variation due to $y$, and Eq.(14) are used to solve this problem.

The prejection of element coordinates $x(z)$ and $y(z)$ on $x z$ and $y z$ planes can be written as

$$
\begin{align*}
& x(z)=x_{l}+x_{a}  \tag{15}\\
& y(z)=y_{l}+y_{d}
\end{align*}
$$

The components $x_{l}$ and $y_{l}$ are linear, while $x_{d}$ and $y_{a}$ are nonlinear. The components $x_{a}$ and $y_{a}$ are approximated by a cubic polynomial in $z$. Thus,

$$
\left\{\begin{array}{l}
x_{a}  \tag{16}\\
y_{a}
\end{array}\right\}=\{N]\{d\}
$$

where $[N]$ is the matrix of shape function and (d) is the local degree of freedom.
The global equilibrium condition $\delta \boldsymbol{\|}=0$ yields the equilibrium equations for the entire system which is

$$
\begin{equation*}
\left\{\frac{\partial \Pi}{\partial D}_{i}\right\}=\{0\} \tag{17}
\end{equation*}
$$

This is a system of nonlinear equations. Then, by the Newton-Raphson iterative procedure, one can write the incremental equation as

$$
\begin{equation*}
\left.\left(K_{N L}\right)\right](\Delta D)=-|R| \tag{18}
\end{equation*}
$$

consider the $k^{\text {th }}$ element, the contribution to the square matrix $\left(K_{N L}\right)$ and to the ector $\{R$ \} are as follows

$$
\begin{align*}
& {\left[\frac{\partial^{2} \pi_{k}}{\partial d_{i} \partial d_{j}}\right]=\int_{0}^{h} \frac{(N \cdot)^{T} T\left(N^{\cdot}\right)}{\left(1+x^{\prime 2}+y^{\prime 2}\right)^{3 / 2}} d z}  \tag{19}\\
& {\left[\frac{\partial R_{k}}{\partial d_{i}}\right]=\int_{0}^{h}\left[\frac{(N \cdot)^{T} T}{\left(1+x^{\prime 2}+y^{\prime 2}\right)^{\frac{1}{2}}}\left[\begin{array}{l}
x^{\prime} \\
y^{\prime}
\end{array}\right]-(N)^{T}\left(\begin{array}{l}
P_{x} \\
P_{y}
\end{array}\right]\right] d z} \tag{20}
\end{align*}
$$

where $h$ is an element height. Eqs. (19) and (20) are evaluated by using Gaussian quadrature numerical integration. The iteration process begins with assuming an initial value of ( $D_{0}$ l. From Eq. (14) obtain tension T, then substitute $T$ into Eqs. (19) and (20) and to obtain the new values $\left\{D_{1}\right\}$. Use $\left\{D_{1}\right\}$ as the initial value and repeat the iteration until convergence criterion is satisfied.

## 5. NUMERICAL EXAMPLE

An example of undersea buggy given by kef. 131 is used for demonstration. The parameters used are listed in Table 1 and plan view of ship and buggy is shown in Fig. 2.

## Table 1 Program Parametery

| Water depth, $m$ | 183 |
| :--- | :--- |
| Ship-to-buggy distance along seabed ( $R$ ), $m$ | 229 |
| Length of cable, $m$ | 305 |
| Net weight of cable, $N / m$ | 12.3 |
| Cable diameter, $m$ | 0.023 |
| Surface current velocity in x-direction, $k m / h r$ | 3.7 |
| Normal drag coefficient | 1.0 |
| Tangential drag coefficient | 0.005 |

By using 10 equal finite elements, the results of total cable length and the variation of cable tension for various horizontal angle $\psi_{0}$ (angle made by current with ship-to-buggy) are given in Table 2 and Fig. $3^{\circ}$ respectively. A godd agreements is obtained.


Fig. 2 Plan view of ship and buggy end




Fig. 3 Voriation of tension with current direction

$$
H=183 \mathrm{M}, R=229 \mathrm{M}
$$

Table 2 Comparison of cable length, $m$

| Ref.(3) | This study |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\psi_{0}=0^{\circ}$ | $30^{\circ}$ | $60^{\circ}$ | $90^{\circ}$ | $120^{\circ}$ | $150^{\circ}$ | $180^{\circ}$ |  |
| 305 | 315 | 317 | 313 | 319 | 321 | 329 | 333 |  |

A variational formulation has been introduced for atatic analysis of marine cables in three dimensional space. A finite element method was developed to solve the problem. The method is suitable for marine cables having known top tension and unknown total cable length.

## REFERENCES:

[1] Berteaux, H.O. (1976), Buoy Engineering, John Wiley 8 Sons, New York, NY, pp. 97-134.
[2] Chucheepsakul, S. and Huang, T. (1990), "Static Equilibrium of Marine Cables by a Variational Method," Proceedings of the First Pacific/Asia Conference on Offshore Mechanics and Nrctic Enginecring, Scoul, Kurea, pp. 329-334.
[3] De Zoysa, A.P.K. (1978), "Steady - State Analysis of Undersea Cables," Ocean Engineering, Vol. 5, pp. 209-223.
[4] Felippa, C.A. (1974), "Finite Element Analysis of Three Dimensianal Cable Structures," Proceedings of the International Conference on Computational Mothods in Nonlinear Mechanics, Austin, Texas, pp. 311-324.
[5] McAllister, T. and Leonard, J. (1986), "Program for Three-Dimensional steadyState Analysis of Grapnel Lines," ASME Proceedings of the First OMAE Specialty Symposium on offshore and Arctic Frontiers, pp. 285-290.
[6] Subwonglee, S., "Static Analysis of Cables in Two and Three Dimensions by a Variational Method," Master of Engineering Thesis, Dept. of Civil Eng., KMIT Thonburi, 1989.

